

GENERAL NOTES:

BUILDING CODE:

A. ALL CONSTRUCTION SHALL CONFORM WITH THE PROVISIONS OF THE 2009 INTERNATIONAL BUILDING CODE.

DESIGN LOADS:

A. DESIGN DEAD LOADING IS AS FOLLOWS:

VESTIBULE ROOF:	
ROOFING	1 PSF
INSULATION	2 PSF
3/4" SHEATHING	3 PSF
RAFTERS	3 PSF
CEILING	3 PSF
MISCELLANEOUS	3 PSF
TOTAL	15 PSF

B. DESIGN LIVE LOADING IS AS FOLLOWS:

ROOF	20 PSF
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C. DESIGN SNOW LOADING IS AS FOLLOWS:

SNOW	USE 30 PSF (NOT REDUCIBLE) Pg = 30 PSF Is = 1.0, OCCUPANCY CATEGORY IV Ce = 1.0, EXPOSURE C Ct = 1.1, UNHEATED TRUSS SPACE Pf = 23.1 PSF
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D. DESIGN LATERAL LOADING IS AS FOLLOWS:

WIND	90 MPH (3-SECOND GUST), EXPOSURE C Iw = 1.0, OCCUPANCY CATEGORY II
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E. THE CONTRACTOR IS COMPLETELY RESPONSIBLE FOR THE METHODS OF CONSTRUCTION AND SHALL PROVIDE ALL GUYS, BRACING AND SHORING REQUIRED TO ACCOMMODATE ALL INTERIM LOADING CONDITIONS THROUGHOUT THE CONSTRUCTION PHASE.

F. WEIGHT OF EQUIPMENT SHOWN ON THE STRUCTURAL DRAWINGS HAS BEEN CONSIDERED IN THE DESIGN OF THE FRAMING. ANY ADDITIONAL EQUIPMENT NOT SHOWN ON THE STRUCTURAL DRAWINGS AND EXCEEDING 300 POUNDS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER FOR APPROVAL PRIOR TO INSTALLATION.

GENERAL:

- A. IT IS THE CONTRACTOR'S RESPONSIBILITY TO SATISFY HIMSELF AS TO THE LOCATION OF ANY UTILITIES IN THE IMMEDIATE VICINITY OF CONSTRUCTION SO AS TO PREVENT DAMAGE TO THEM.
- B. CONSULT THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR VERIFICATION OF LOCATION AND DIMENSION OF CHASES, INSERTS, OPENINGS, SLEEVES, WASHERS, DRIPS, REVEALS, DEPRESSIONS AND OTHER PROJECT REQUIREMENTS.
- C. THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS BEFORE PROCEEDING WITH CONSTRUCTION. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT.
- D. ALL WORK SPECIFIED HEREIN SHALL BE INSPECTED IN ACCORDANCE WITH THE BUILDING CODE AND ALL LOCAL ORDINANCES.
- E. SHOP DRAWINGS FOR ALL STRUCTURAL ELEMENTS SHOWN ON THE CONTRACT DOCUMENTS MUST BE SUBMITTED BY THE GENERAL CONTRACTOR.
- F. ANY REVIEW OF STRUCTURAL ITEM SHOP DRAWINGS BY THE STRUCTURAL ENGINEER IS FOR THE GENERAL CONFORMANCE WITH THE DESIGN CONCEPT AS PRESENTED BY THE CONTRACT DOCUMENTS. NO DETAILED CHECK OF QUANTITIES OR DIMENSIONS WILL BE MADE.
- K. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO EXAMINE THE EXISTING BUILDING DURING THE COURSE OF CONSTRUCTION AND IMMEDIATELY ADVISE THE ARCHITECT OF ANY AREAS WHERE THE STRUCTURE EXHIBITS DISTRESS OR FAILURE.
- L. THE CONTRACTOR SHALL EXERCISE CARE DURING DEMOLITION AND CONSTRUCTION AS REQUIRED TO MAINTAIN THE STABILITY OF EXISTING CONSTRUCTION. EXISTING WALLS ARE DEPENDENT UPON THE EXISTING FLOORS AND ROOFS FOR STABILITY. ALL FLOORS AND ROOFS, WHICH HAVE COLLAPSED OR ARE TO BE DEMOLISHED, MUST BE REPLACED WITH ADEQUATE BRACING AS REQUIRED TO MAINTAIN EXISTING CONSTRUCTION.
- M. THE CONTRACTOR SHALL EXERCISE CARE DURING THE EXCAVATION AND CONSTRUCTION OF NEW FOOTINGS AND UNDERPINNING AT EXISTING WALLS AS REQUIRED TO MAINTAIN STABILITY OF THE WALL. THE EARTH AGAINST THE BASE OF THE WALL PROVIDES LATERAL BRACING FOR THE WALL, ESPECIALLY AT RETAINING WALLS. AS EARTH IS REMOVED, BRACE BASE AS REQUIRED.
- N. THE CONTRACTOR SHALL PROVIDE ALL SHORING, NEEDLING AND BRACING AS REQUIRED TO SUPPORT THE EXISTING STRUCTURE. THE CONTRACTOR SHALL EXAMINE THE EXISTING STRUCTURE TO DETERMINE THE EXTENT OF NECESSARY SHORING, NEEDLING AND UNDERPINNING. THE CAPACITY AND METHOD USED FOR SHORING AND NEEDLING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- J. THE CONTRACTOR IS ADVISED THAT ALL PLANS, DIMENSIONS, AND DETAILS DEPICT FIELD CONDITIONS AS SHOWN. MINOR VARIATIONS ARE TO BE EXPECTED AND ANY DEVIATIONS FROM THE CONTRACT DOCUMENTS SHALL BE APPROVED BY THE ARCHITECT IN WRITING PRIOR TO PROCEEDING.
- K. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE COMPLIANCE WITH ALL OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA) REGULATIONS.

FOUNDATIONS:

- A. FOOTINGS ARE DESIGNED BASED ON A NET ALLOWABLE SOIL BEARING CAPACITY OF 2,000 PSF.
- B. FOOTINGS SHALL BEAR ON NATURAL UNDISTURBED SOIL OR ON CONTROLLED STRUCTURAL FILL.
- C. THE BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 3'-0" BELOW FINISHED GRADE.
- D. THE CONTRACTOR SHALL RETAIN THE SERVICES OF A LICENSED GEOTECHNICAL ENGINEER TO VERIFY THE SOIL BEARING CAPACITY IN THE FIELD DURING CONSTRUCTION.
- E. ALL FILL UNDER SLABS ON GROUND SHALL BE COARSE GRANULAR MATERIAL COMPACTED TO 95% MAXIMUM DENSITY AS DETERMINED BY ASTM D1557 (MODIFIED PROCTOR)

CONCRETE:

A. ALL CONCRETE SHALL BE STONE AGGREGATE CONCRETE HAVING A MINIMUM COMPRESSIVE STRENGTH AS LISTED BELOW:

FOOTINGS:	3000 PSI	MAXIMUM SLUMP = 4"	NO AIR
INTERIOR SLABS:	4000 PSI	MAXIMUM SLUMP = 4"	NO AIR
EXTERIOR SLABS:	4000 PSI	MAXIMUM SLUMP = 4"	AIR
WALLS:	4000 PSI	MAXIMUM SLUMP = 4"	AIR

B. ALL CONCRETE EXPOSED TO WEATHER SHALL HAVE AN AIR ENTRAINMENT OF 6.5% ± 1.5%.

C. NO ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL BE PERMITTED.

D. UNLESS OTHERWISE NOTED, MAXIMUM AGGREGATE SIZE FOR CONCRETE SHALL BE 1".

E. ALL CONCRETE SHALL CONTAIN A WATER REDUCING ADMIXTURE. MAXIMUM SLUMPS INDICATED ARE PRIOR TO THE ADDITION OF MID-RANGE WATER REDUCING ADMIXTURE OR SUPER PLASTICIZER.

F. PROVIDE ACI CLASS B LAP SPLICE AT ALL BAR LAPS WITH A MINIMUM LAP OF 24".

G. ALL REINFORCING BARS SHALL CONFORM TO ASTM A-615 GRADE 60.

H. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185 GRADE 65. ALL WELDED WIRE FABRIC SHALL BE IN SHEETS AND SUPPORTED ON CHAIRS.

MASONRY:

A. CONCRETE MASONRY UNITS SHALL HAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1,900 PSI (NET AREA) FOR F M = 1,500 PSI. TYPE "S" PORTLAND CEMENT/LIME MORTAR SHALL BE USED FOR ALL MASONRY.

B. MASONRY BEARING WALLS SHALL CONSIST OF STANDARD HOLLOW NORMAL WEIGHT UNITS CONFORMING TO ASTM C 90, UNLESS OTHERWISE NOTED.

C. ALL SOLID CMU IS TO BE 100% SOLID CMU OR HOLLOW CMU WITH ALL CELLS FILLED 100% SOLID WITH PEA GRAVEL CONCRETE WITH F C = 3,000 PSI OR GROUT CONFORMING TO ASTM C 476.

D. LAP ALL REINFORCING 48 BAR DIAMETERS MINIMUM.

E. ALL MASONRY WALLS SHALL BE REINFORCED WITH NO. 3 GAGE, LADDER OR TRUSS-TYPE HOT DIPPED GALVANIZED JOINT REINFORCEMENT SPACED VERTICALLY AT 16" O/C, UNLESS NOTED OTHERWISE. LAP REINFORCEMENT 6" MINIMUM.

F. ALL FILL FOR MASONRY WALLS SHALL BE GROUT CONFORMING TO A.S.T.M. C476. FILL SHALL BE PLACED IN 5'-0" MAXIMUM LIFTS.

STRUCTURAL STEEL:

A. ALL STRUCTURAL AND MISCELLANEOUS STEEL SHALL CONFORM TO THE THIRTEENTH EDITION OF THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (AISC 360-09)" AND ALL ITS SUPPLEMENTS, AND TO THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES (AISC 303-05)."

B. PROVIDE STRUCTURAL STEEL FRAMING SECTIONS AS FOLLOWS:
WIDE FLANGE SHAPES: ASTM A-992, FY = 50,000 PSI
CHANNELS, ANGLES, PLATES, BARS, RODS: ASTM A-36, FY = 36,000 PSI
HSS SHAPES: ASTM A500 GRADE B, FY = 46,000 PSI.
PIPES: GRADE B, FY = 35,000 PSI.

C. BOLTS SHALL COMPLY WITH ASTM A490 OR A325. BOLTS SHALL BE A MINIMUM 3/4" DIAMETER, UNLESS OTHERWISE NOTED. NUTS AND WASHERS SHALL BE COMPATIBLE WITH THE GRADE, HOLE SIZE, CONNECTION TYPE AND INSTALLATION METHOD AS INDICATED IN THE "SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS".

D. WELDING ELECTRODES SHALL BE E70XX. WELDING SHALL BE COMPLETED BY AWS CERTIFIED WELDERS AND SHALL CONFORM TO AWS AMERICAN WELDING SOCIETY CODE FOR BUILDINGS, AWS D1.1.

E. PROVIDE 1/2" (MINIMUM) CAP PLATE AT ALL COLUMNS, UNLESS NOTED OTHERWISE.

F. THE USE OF A GAS-CUTTING TORCH IN THE FIELD FOR CUTTING HOLES OR FOR CORRECTING FABRICATION ERRORS WILL NOT BE PERMITTED ON NEW STRUCTURAL FRAMING MEMBERS EXCEPT WITH THE WRITTEN APPROVAL OF THE ENGINEER FOR EACH SPECIFIC CONDITION.

G. ALL STRUCTURAL STEEL SHALL BE SHOP PAINTED WITH AN CORROSION RESISTANT PRIMER.

H. ALL EXTERIOR STRUCTURAL STEEL AND STEEL EXPOSED TO WEATHER, INCLUDING BOLTS AND ANCHOR BOLTS, SHALL BE HOT-DIP GALVANIZED CONFORMING TO ASTM A123.

I. ALL ABRADED AREAS AND FIELD WELDS SHALL BE FIELD COATED WITH A COLD GALVANIZING COMPOUND CONFORMING TO ASTM A780.

PREFABRICATED WOOD TRUSSES:



WOOD FRAMING:

A. ALL WOOD FRAMING SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH "THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", LATEST EDITION, AS PUBLISHED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.

B. ALL CONVENTIONAL LUMBER SHALL BE NO. 1/NO. 2 SPRUCE/PINE/FIR 19% MAXIMUM MOISTURE CONTENT OR BETTER. THE MINIMUM DESIGN VALUES SHALL BE AS FOLLOWS:

F _b = 875 PSI
F _v = 135 PSI
E = 1,400,000 PSI

C. ALL ENGINEERED LUMBER SHALL BE MANUFACTURED BY WEYERHAEUSER (OR APPROVED EQUAL) AND HAVE THE MINIMUM DESIGN VALUES AS LISTED BELOW:

1.90E MICROLAM LVL: F _b = 2,600 PSI F _v = 285 PSI E = 1,900,000 PSI	1.8E PARALLAM PSL (COLUMN): F _b = 2,400 PSI F _v = 190 PSI E = 1,800,000 PSI
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D. ALL STEEL TIMBER FASTENINGS AND JOIST HANGERS SHALL BE A MINIMUM OF 18 GAGE GALVANIZED STEEL WITH A RATED LOAD CAPACITY EQUAL TO OR EXCEEDING THE IMPOSED LOADING REQUIREMENTS.

E. NAILING OF ALL FRAMING SHALL MEET THE RECOMMENDED FASTENING SCHEDULE CONTAINED IN THE BUILDING CODE.

F. CUTTING AND NOTCHING OF WOOD FRAMING SHALL ONLY BE PERMITTED WITHIN THE LIMITS PRESCRIBED BY THE BUILDING CODE.

G. ANCHOR ALL PRESSURE TREATED SILL PLATES TO CONCRETE SLABS OR MASONRY WITH GALVANIZED ANCHORS AS INDICATED ON DRAWINGS.

H. ALL DOUBLE JOISTS, HEADERS, OR BEAMS SHALL BE SPIKED TOGETHER WITH (2) ROWS OF 16d NAILS AT 16" O/C, UNLESS OTHERWISE NOTED.

I. ALL BEAMS MADE UP OF 3 OR MORE PLYS SHALL BE BOLTED TOGETHER WITH (2) ROWS OF 1/2" DIAMETER BOLTS AT 32" O/C.

J. OSB SHEATHING AT PITCHED SHINGLED ROOFS SHALL BE ATTACHED WITH 10D NAILS SPACED NOT LESS THAN 6" O/C AT ALL EDGES, AND NOT LESS THAN 8" O/C FOR ALL INTERMEDIATE SUPPORTS. ROOF SHEATHING SHALL BE CONTINUOUS OVER A MINIMUM OF 3 SPANS.

K. ADVANTECH SHEATHING AT FLAT ROOF AREAS SHALL BE ATTACHED WITH 10D NAILS SPACED NOT LESS THAN 6" O/C AT ALL EDGES, AND NOT LESS THAN 8" O/C FOR ALL INTERMEDIATE SUPPORTS. ROOF SHEATHING SHALL BE CONTINUOUS OVER A MINIMUM OF 3 SPANS.

L. WALL SHEATHING SHALL BE ATTACHED WITH 10D NAILS SPACED AT 8" O/C. (MAX.)

SPECIAL INSPECTIONS

A. SPECIAL INSPECTIONS SHALL BE PROVIDED IN ACCORDANCE WITH CHAPTER 17 "STRUCTURAL TESTS AND SPECIAL INSPECTIONS" OF THE 2009 INTERNATIONAL BUILDING CODE.

B. THE THIRD PARTY INSPECTION AGENCY(S) RETAINED TO PERFORM SPECIAL INSPECTIONS DURING CONSTRUCTION, SHALL SUBMIT FOR APPROVAL, TO THE BUILDING OFFICIAL, WRITTEN DOCUMENTATION DEMONSTRATING COMPETENCY TO PERFORM THE SPECIAL INSPECTION(S) THEY HAVE BEEN RETAINED TO COMPLETE.

C. IT SHALL BE THE RESPONSIBILITY OF THE SPECIAL INSPECTION AGENCY(S) TO REVIEW, UNDERSTAND AND PERFORM ALL REQUIRED SPECIAL INSPECTIONS AT THE FREQUENCY DEFINED IN CHAPTER 17.

D. THE SPECIAL INSPECTION AGENCY(S) SHALL KEEP REPORTS OF ALL INSPECTIONS AS RECORD, AND SUBMIT COPIES OF ALL REPORTS TO THE BUILDING OFFICIAL AND WOLF CONSULTING ENGINEERS PER SECTION 1704.1.2.

REQUIRED SPECIAL INSPECTIONS PER IBC CHAPTER 17	
	REQUIRED
CONCRETE CONSTRUCTION (TABLE 1704.4):	
VERIFICATION OF MATERIALS: (REINFORCING STEEL, DESIGN MIX)	X
FRESH CONCRETE TESTS: (STRENGTH, SLUMP, AIR CONTENT)	X
SOILS (TABLE 1704.7):	
SOIL BEARING CAPACITY	X
EXCAVATION DEPTH	X

STRUCTURAL ABBREVIATIONS:

ARCH	ARCHITECTURAL
CMU	CONCRETE MASONRY UNIT
CONT	CONTINUOUS
DWG	DRAWING
EOA	EDGE OF ANGLE
EOD	EDGE OF DECK
EOJ	END OF JOIST
EOS	EDGE OF SLAB
EQ	EQUAL
EX	EXISTING
E/W	EACH WAY
FRT	FIRE RETARDANT TREATED
FTG	FOOTING
GA	GAUGE
MAX	MAXIMUM
MCJ	MASONRY CONTROL JOINT
MIN	MINIMUM
O/C	ON CENTER
PAF	POWDER ACTUATED FASTENERS
PT	PRESSURE TREATED
SIM	SIMILAR
TOS	TOP OF STEEL
TOW	TOP OF WALL
TYP	TYPICAL

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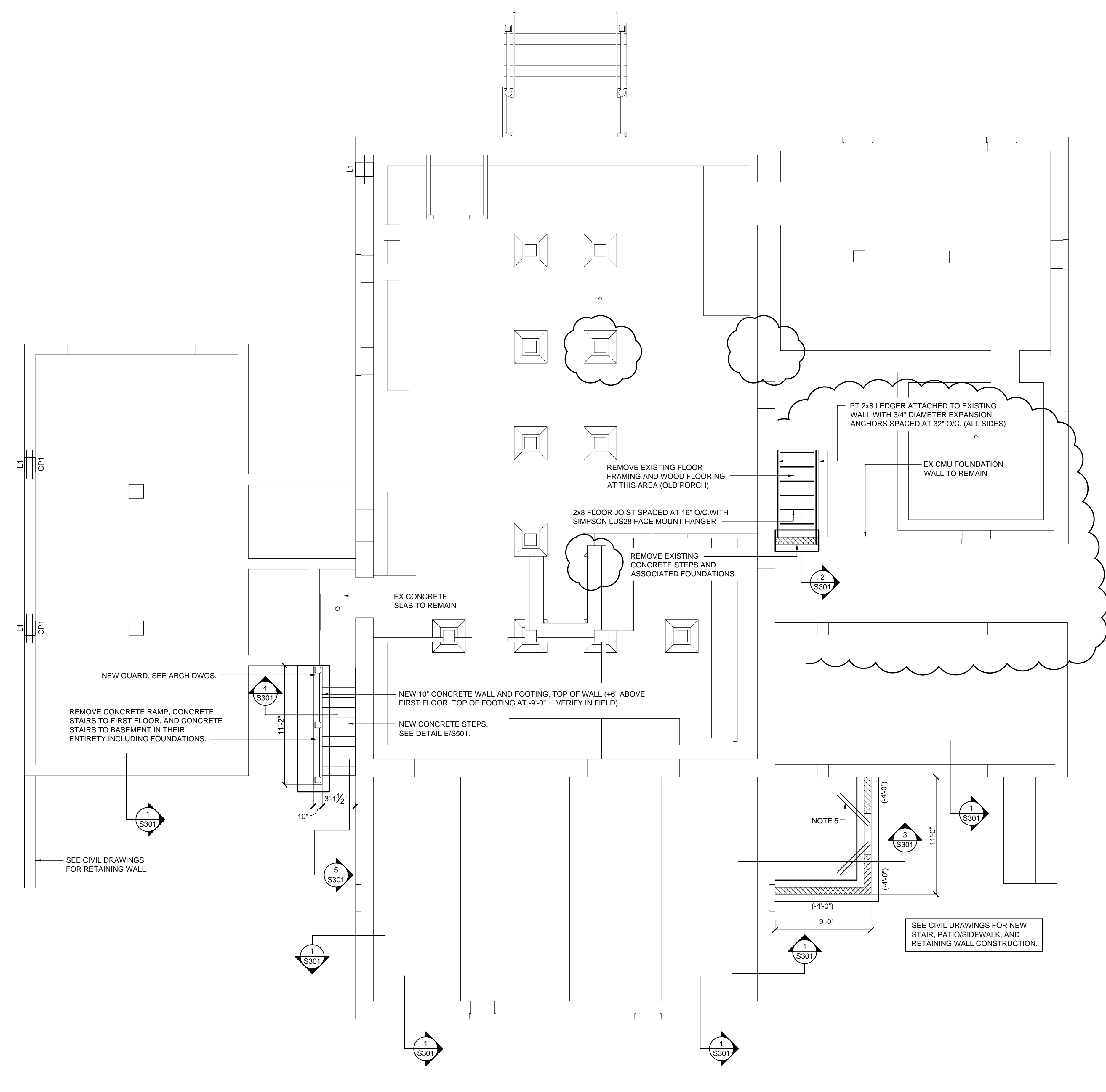
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FOUNDATION PLAN

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FOUNDATION PLAN NOTES:

- FIRST FLOOR ELEVATION = 0'-0" (570.72' FROM SITE DRAWINGS).
- THE ELEVATION OF THE TOP OF FOOTINGS ARE NOTED (-'X'-X") ON THE PLAN INDICATING THE DIMENSION BELOW THE REFERENCED FLOOR ELEVATION.
- DIMENSIONS NOTED ON PLAN ARE TO THE FACE OF THE FOUNDATION WALL OR TO FOOTING CENTERLINES.
- ALL NEW INTERIOR SLAB-ON-GRADE AREAS SHALL BE 4" THICK REINFORCED WITH 6"x6" - W2.1xW2.1 WELDED WIRE FABRIC (W.W.F.) PLACED ON 10 MIL POLYETHYLENE VAPOR BARRIER OVER 4" CRUSHED 2B STONE SUBBASE.
- PROVIDE RE-ENTRANT BARS LOCATED IN SLAB-ON-GRADE. SEE DETAIL C/S501. LOCATIONS OF ALL RE-ENTRANT BARS ARE NOT SHOWN ON PLAN.



FOUNDATION AND FIRST FLOOR FRAMING PLAN
 SCALE: 3/16" = 1'-0"
 NORTH

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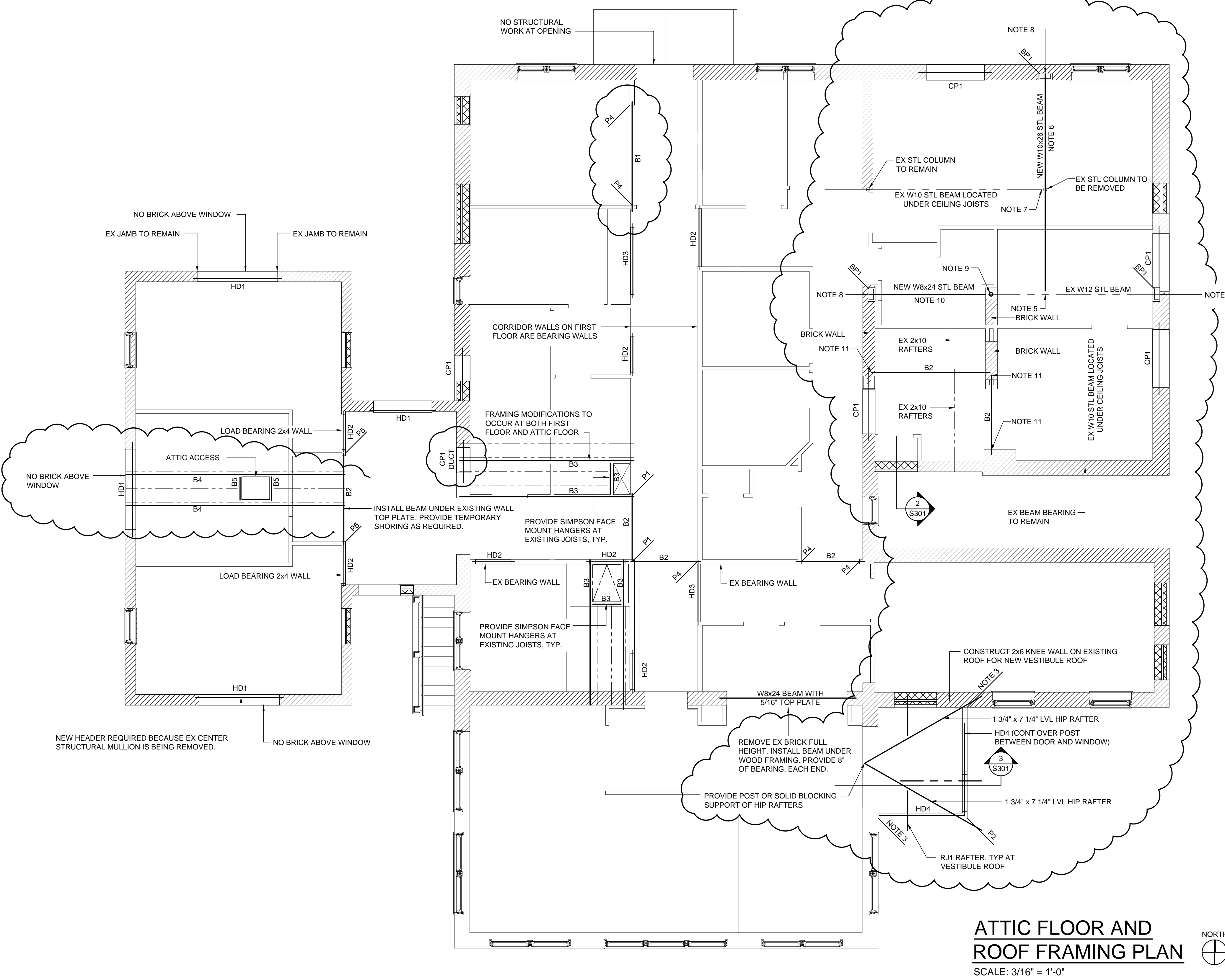
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ROOF FRAMING PLAN

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AFTER DEMOLITION OF FINISHES AND PRIOR TO SUBMITTING STEEL SHOP DRAWINGS FOR REVIEW AND APPROVAL, THE STEEL DETAILER SHALL VISIT THE SITE TO RECORD AND COORDINATE EXISTING CONDITIONS.



ROOF FRAMING PLAN NOTES:

- SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL MASONRY WORK. NON-STRUCTURAL MASONRY INFILL WORK IS NOT DETAILED ON STRUCTURAL DRAWINGS.
- SEE ARCHITECTURAL DRAWINGS FOR ALL ROOF SLOPES.
- ATTACH END STUD TO EXISTING BRICK WALL WITH 1/2" DIAMETER EXPANSION ANCHORS SPACED AT 32" O.C.
- SHORE BEAM. REMOVE MASONRY UNDER BEAM, AND CONSTRUCT CONCRETE BEARING BLOCK. SEE SECTION 11/S301.
- PROVIDE FULL DEPTH DOUBLE ANGLE L3 1/2 x 3 1/2 x 5/16 CONNECTION FROM NEW W10 TO EXISTING W12 BEAM. FIELD WELD CONNECTION ANGLES.
- STEEL BEAM REPLACES THE EXISTING WOOD BEARING WALL LOCATED ON THE FIRST FLOOR. LOCATE BEAM TIGHT TO UNDERSIDE OF EXISTING CEILING JOIST FRAMING. INSTALL 2x6 WOOD PLATE TO TOP OF BEAM WITH 1/2" DIAMETER BOLTS SPACED AT 24" O.C., STAGGERED.
- TEMPORARY SHORE END OF BEAM. REMOVE EXISTING STEEL POST, AND PREPARE FOR INSTALLATION OF NEW W10. PROVIDE FULL DEPTH DOUBLE ANGLE L3 1/2 x 3 1/2 x 5/16 CONNECTION FROM NEW W10 TO EXISTING W10 BEAM. FIELD WELD CONNECTION ANGLES.
- REMOVE EXISTING BRICK AND INSTALL NEW CONCRETE BEARING BLOCK. SEE SECTION 11/S301.
- NEW 4" (4.5" O.D.) SCH 40 STEEL PIPE COLUMN. BEAR ON NEW CONCRETE BEARING BLOCK LOCATED AT TOP OF STONE BASEMENT WALL AND EXTEND UP TO SUPPORT EXISTING W12 BEAM. REMOVE EXISTING BRICK AS NEEDED TO PLACE NEW COLUMN. CONCRETE BEARING BLOCK SHALL BE 8" THICK x 16" x 16" (MIN. SIZE), 3,000 PSL. PROVIDE 5/8" x 10 1/2" x 10 1/2" BASE PLATE AND ATTACH WITH (4) 5/8" DIAMETER EXPANSION ANCHORS. NEW COLUMN SHALL FRAME TO THE UNDERSIDE OF THE W12 WITH A 5/8" THICK CAP PLATE.
- LOCATE BEAM UNDER EXISTING WOOD CEILING JOISTS AND RAFTERS. INSTALL 2x6 WOOD PLATE TO TOP OF BEAM AND ATTACH WITH 1/2" DIAMETER BOLTS SPACED AT 24" O.C., STAGGERED. PROVIDE FULL DEPTH 3/8" SHEAR PLATE CONNECTION OF EAST END TO EXISTING W12.
- POSITION BEAM TIGHT TO UNDERSIDE OF CEILING JOISTS OR RAFTER FRAMING AND PROVIDE 5" (MIN.) BEARING OF LVL BEAM ONTO EXISTING BRICK WALL.

WOOD FRAMING SCHEDULE

MARK	SIZE
HD1	(2) 1 3/4" x 7 1/4" LVL HEADER
HD2	(2) 2x10 HEADER
HD3	(2) 1 3/4" x 9 1/4" LVL HEADER
HD4	(3) 2x10 + (2) 7/16" OSB SPACERS BEAM
RJ1	2x6 AT 16" O.C.
B1	(2) 1 3/4" x 11 7/8" LVL BEAM
B2	(2) 1 3/4" x 9 1/2" LVL BEAM
B3	(1) 1 3/4" x 9 1/2" LVL BEAM
B4	2x8
B5	(2) 2x8
P1	(3) 2x6 POST
P2	(4) 2x6 POST (SEE ARCH PLANS FOR ADDITIONAL DETAIL)
P4	(2) 2x6 POST
P5	(2) 2x4 POST

UNLESS OTHERWISE NOTED, PROVIDE (1) JACK STUD AND (1) KING STUDS ON EACH SIDE OF NEW HEADER OPENINGS. ATTACH JACK STUD TO KING STUD WITH (2) 16D NAILS SPACED AT 8" O.C.

B3 MEMBER MAY NEED RIPPED TO MATCH DEPTH OF EXISTING FRAMING MEMBERS.

ATTIC FLOOR AND ROOF FRAMING PLAN

SCALE: 3/16" = 1'-0"

PLASTER WALL AND CEILING FINISHES ARE BEING REMOVED IN THE ATTIC AREA (SEE ARCH DWGS). WOOD ROOF, CEILING, AND WALL FRAMING IS TO REMAIN. WOLF CONSULTING ENGINEERS SHALL BE CONSULTED IF THE CONTRACTOR WISHES TO REMOVE WOOD FRAMING FOR NEW WORK.

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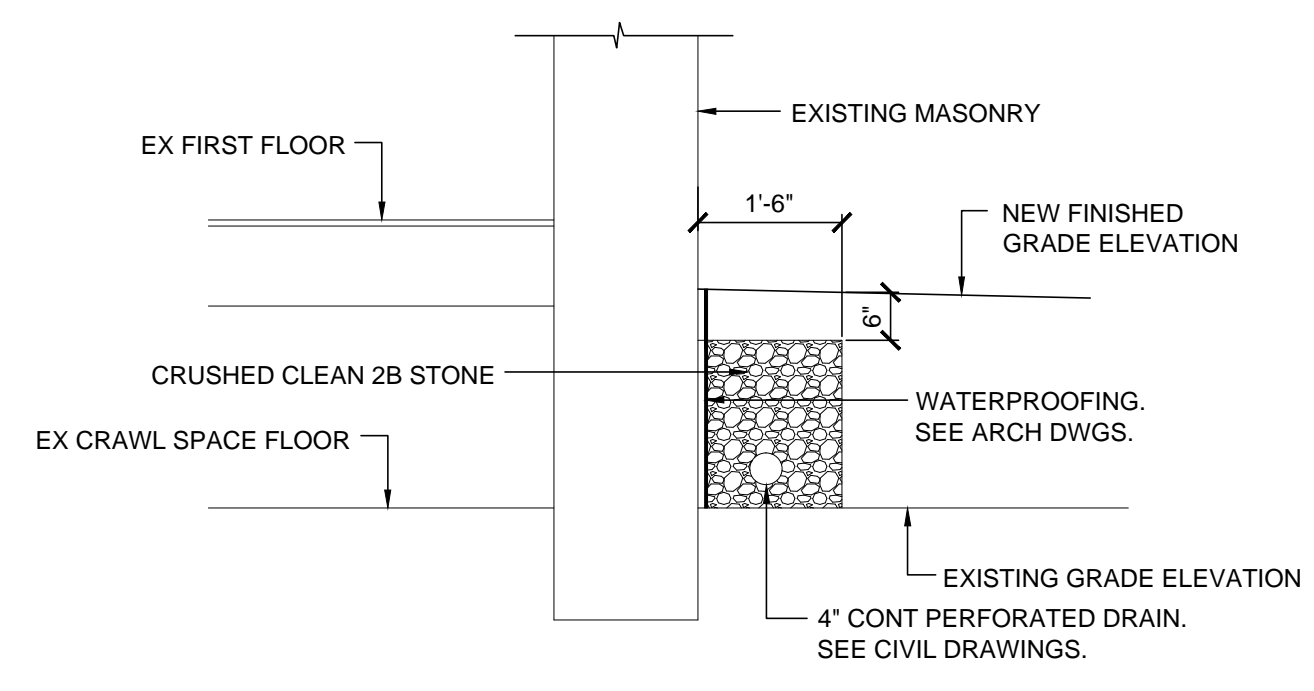
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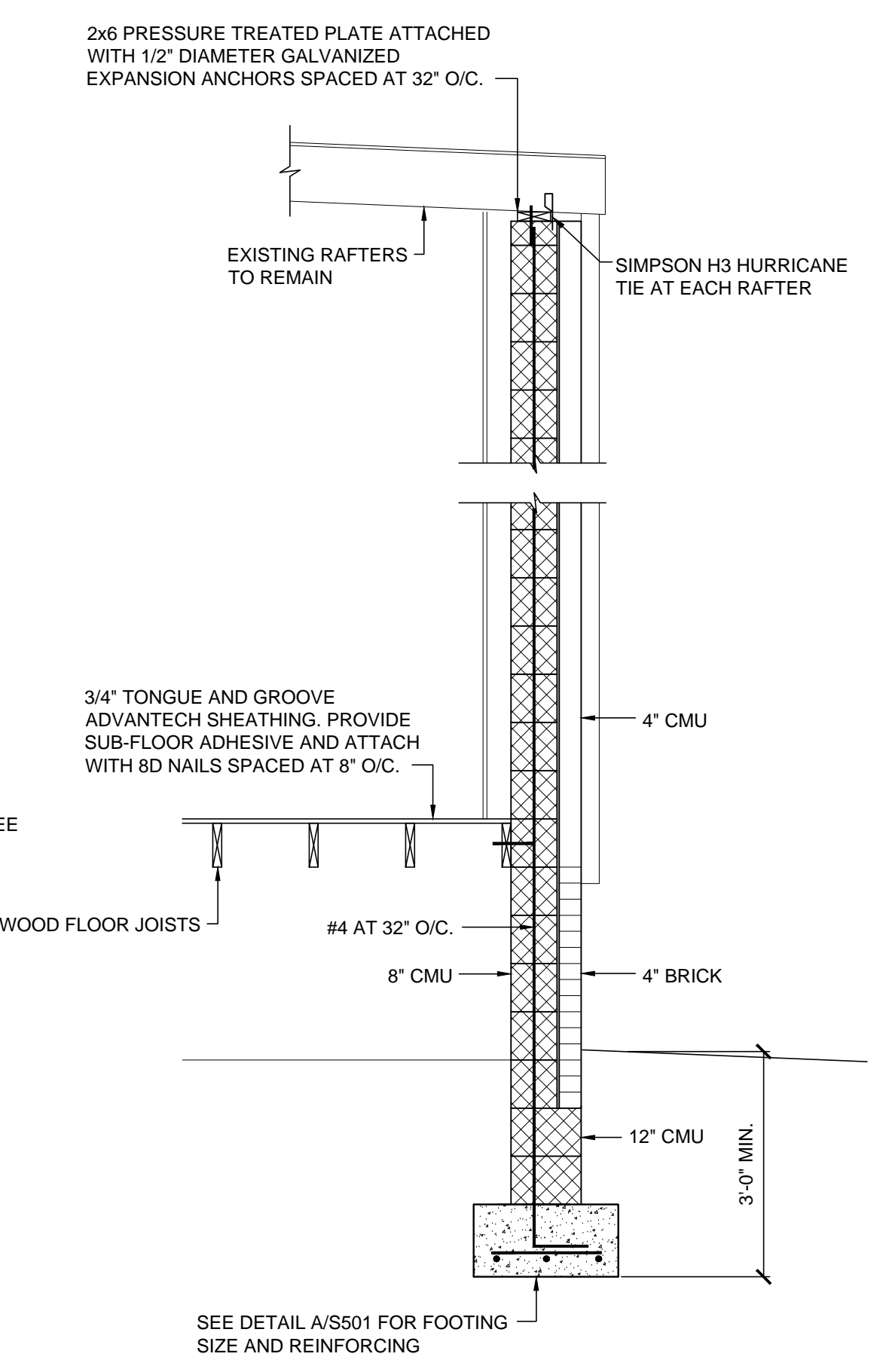
FOUNDATION AND FRAMING SECTIONS

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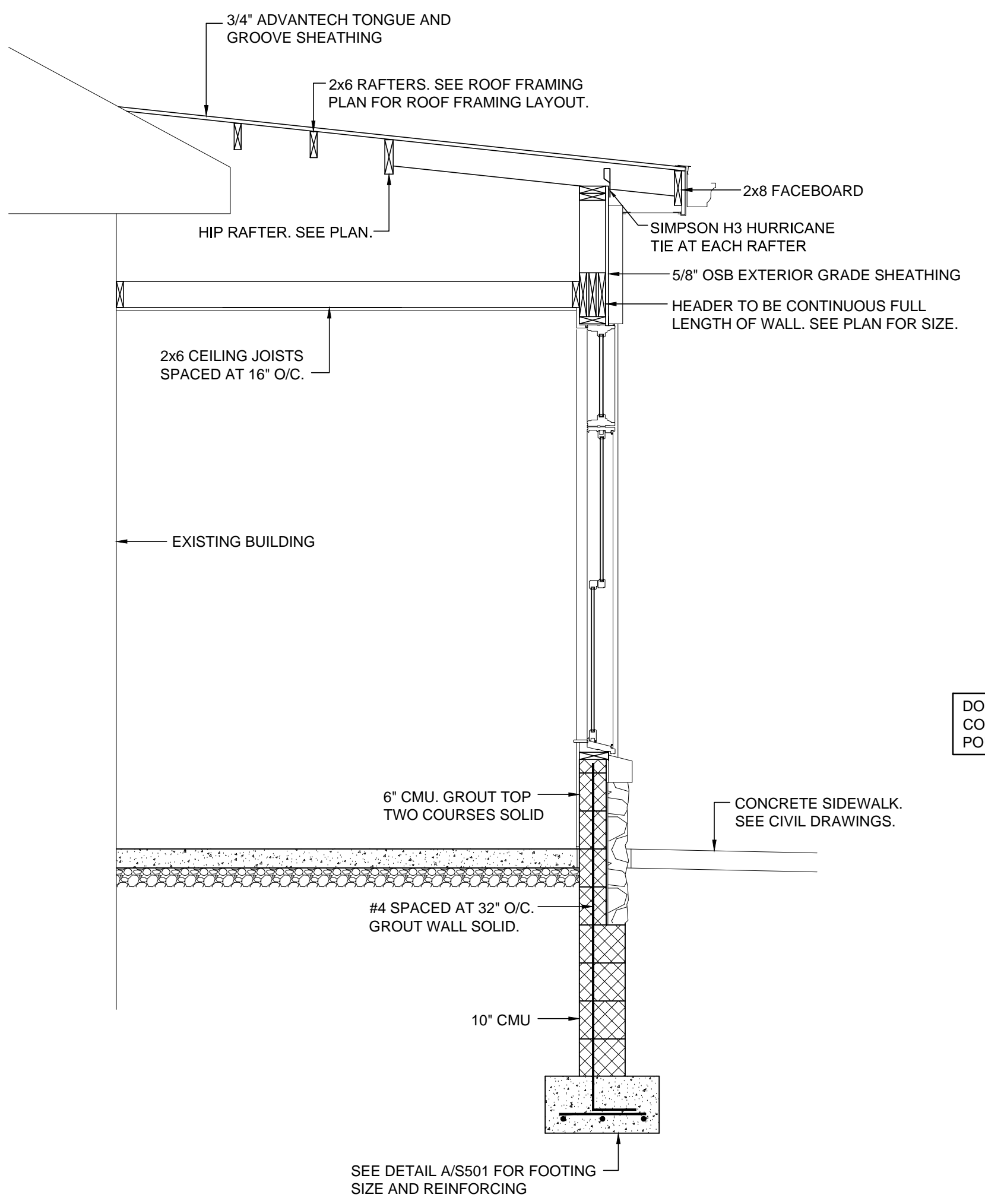
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S301



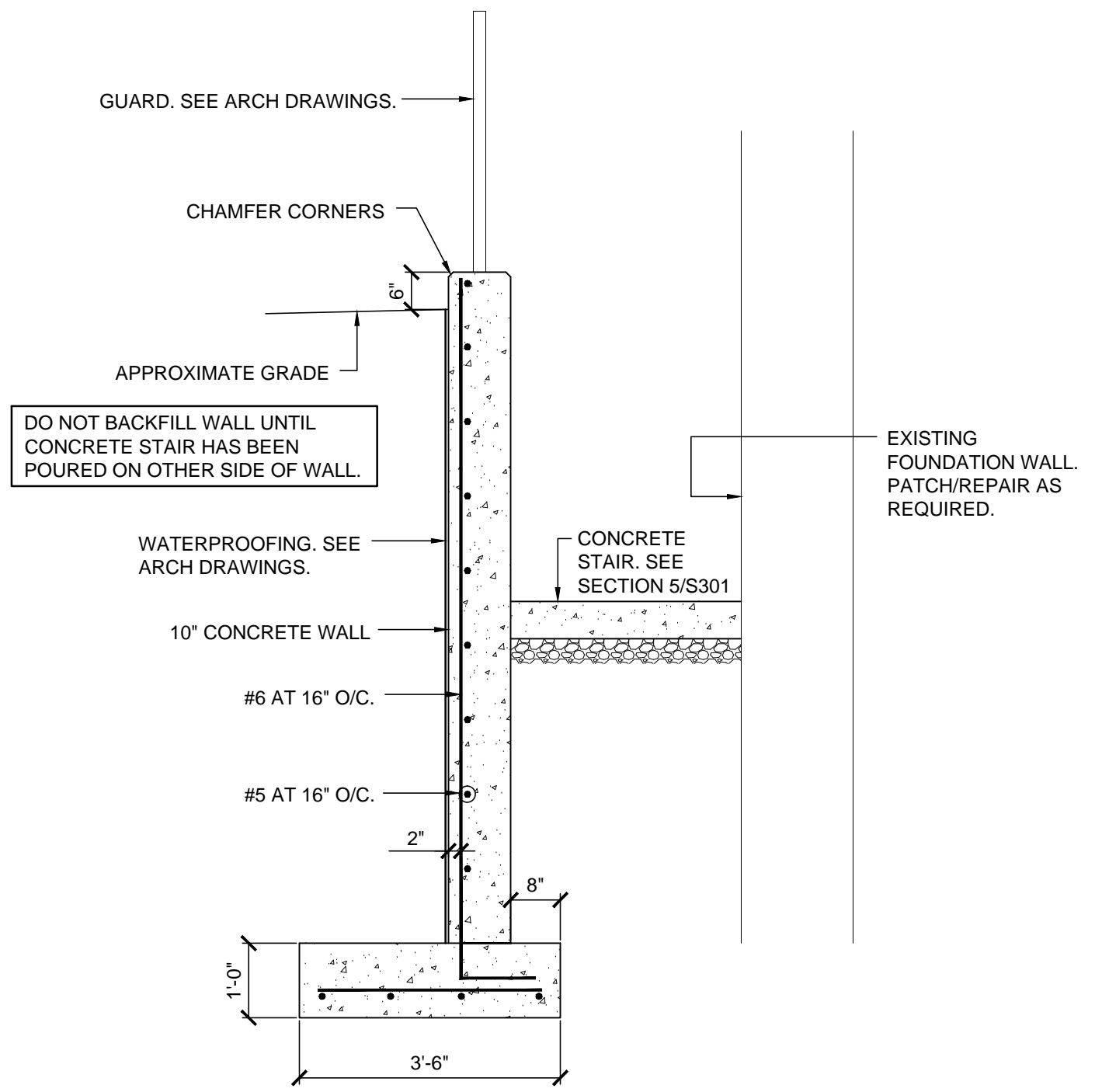
1 FOUNDATION SECTION
 S301 SCALE: 1/2" = 1'-0"



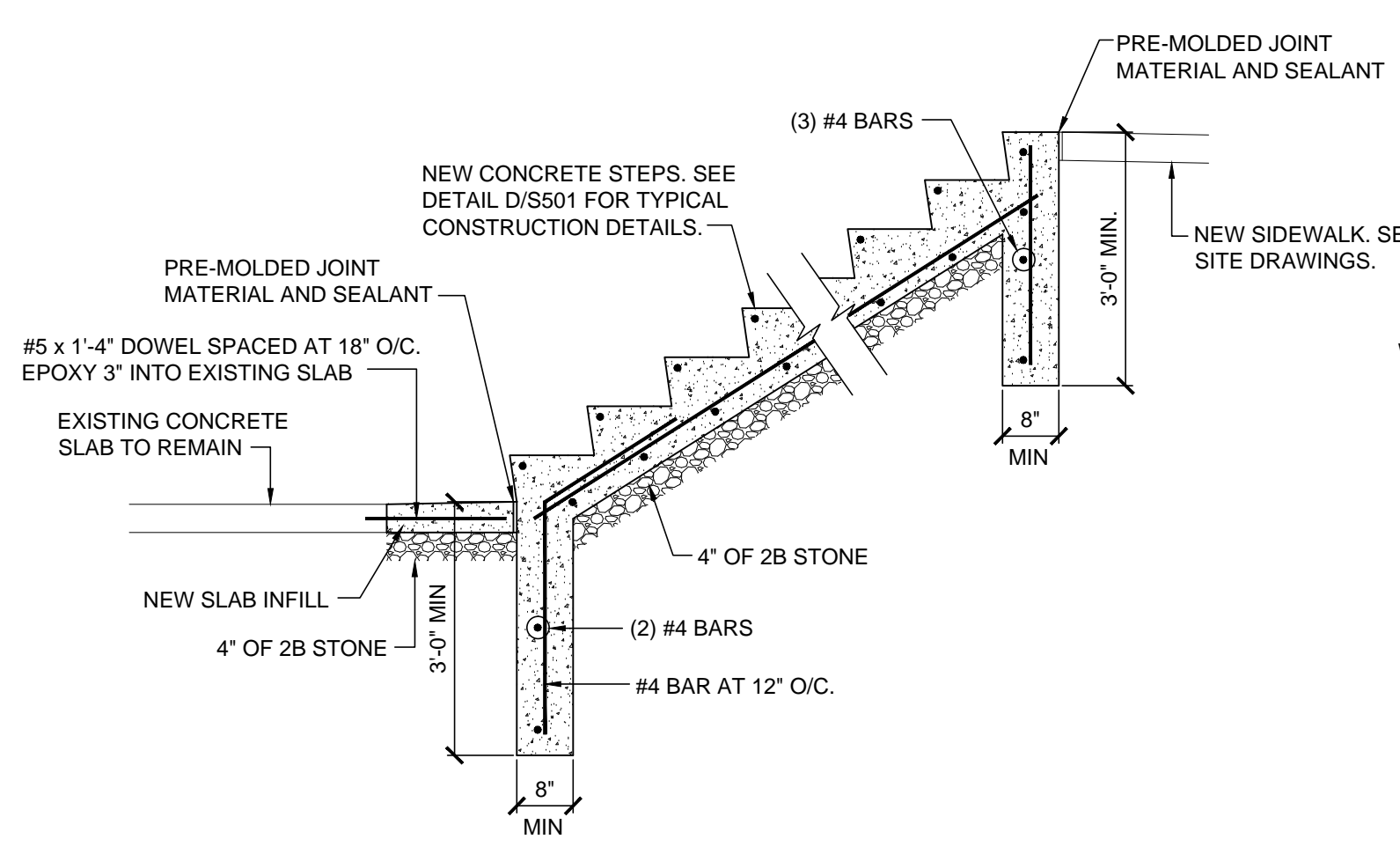
2 WALL SECTION
 S301 SCALE: 1/2" = 1'-0"



3 VESTIBULE SECTION
 S301 SCALE: 1/2" = 1'-0"

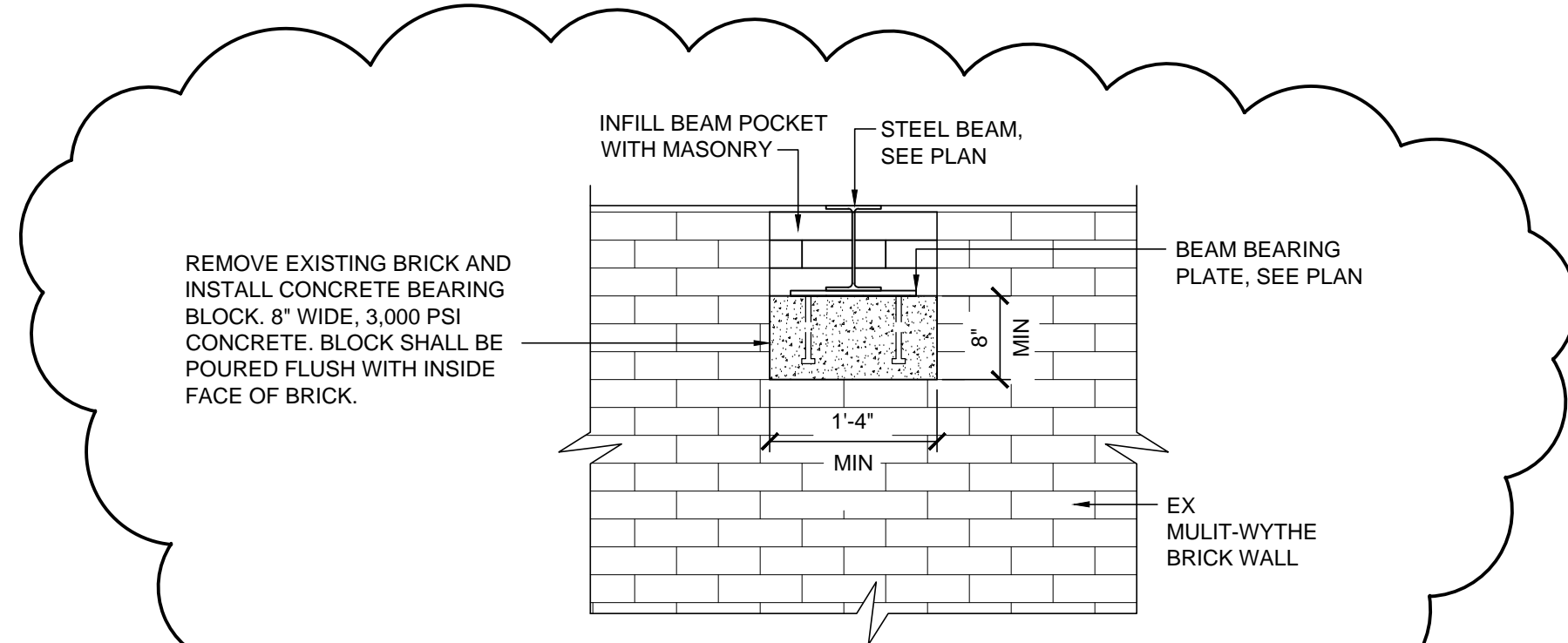


4 RETAINING WALL SECTION
 S301 SCALE: 1/2" = 1'-0"



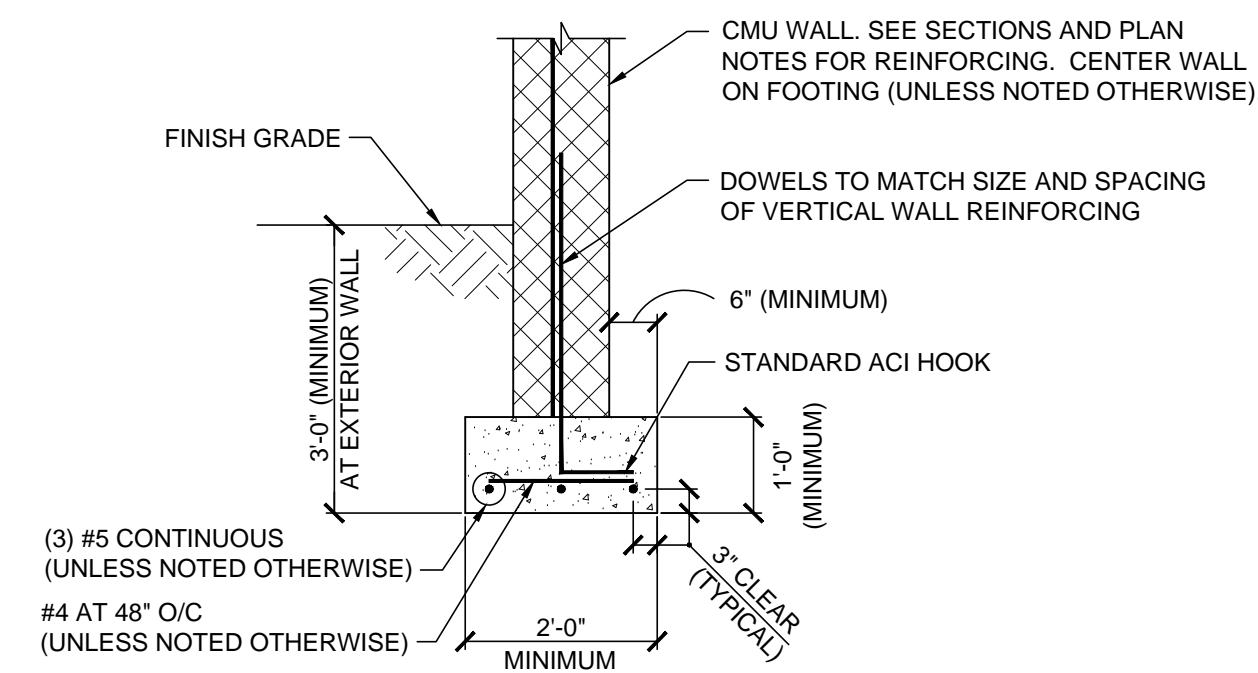
5 SECTION AT CONCRETE STEPS
 S301 SCALE: 1/2" = 1'-0"

SECTIONS DELETED
 6/S301
 7/S301
 8/SS01
 9/S301
 10/S301



11 FRAMING SECTION
 S301 SCALE: 3/4" = 1'-0"

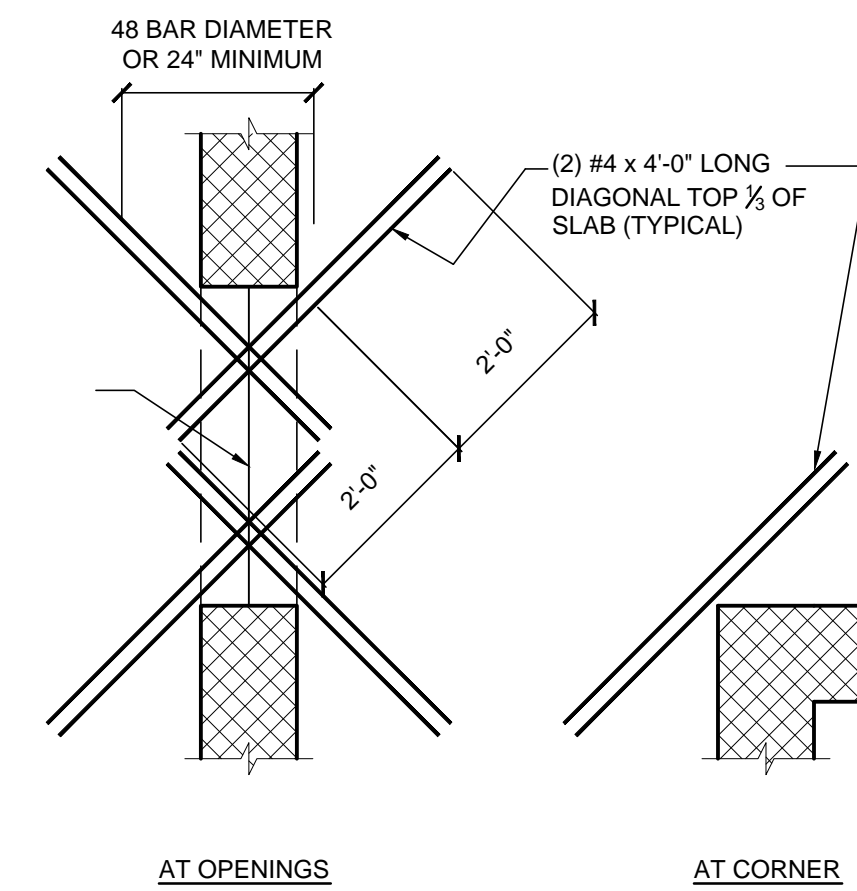
BID SET



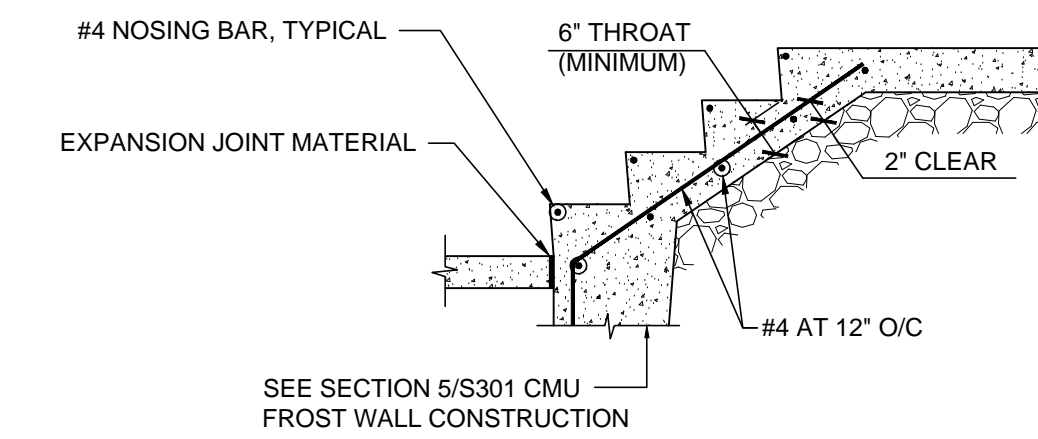
A TYPICAL WALL FOOTING
S501 NOT TO SCALE

NOT USED

B TYPICAL STEPPED FOOTING
S501 NOT TO SCALE



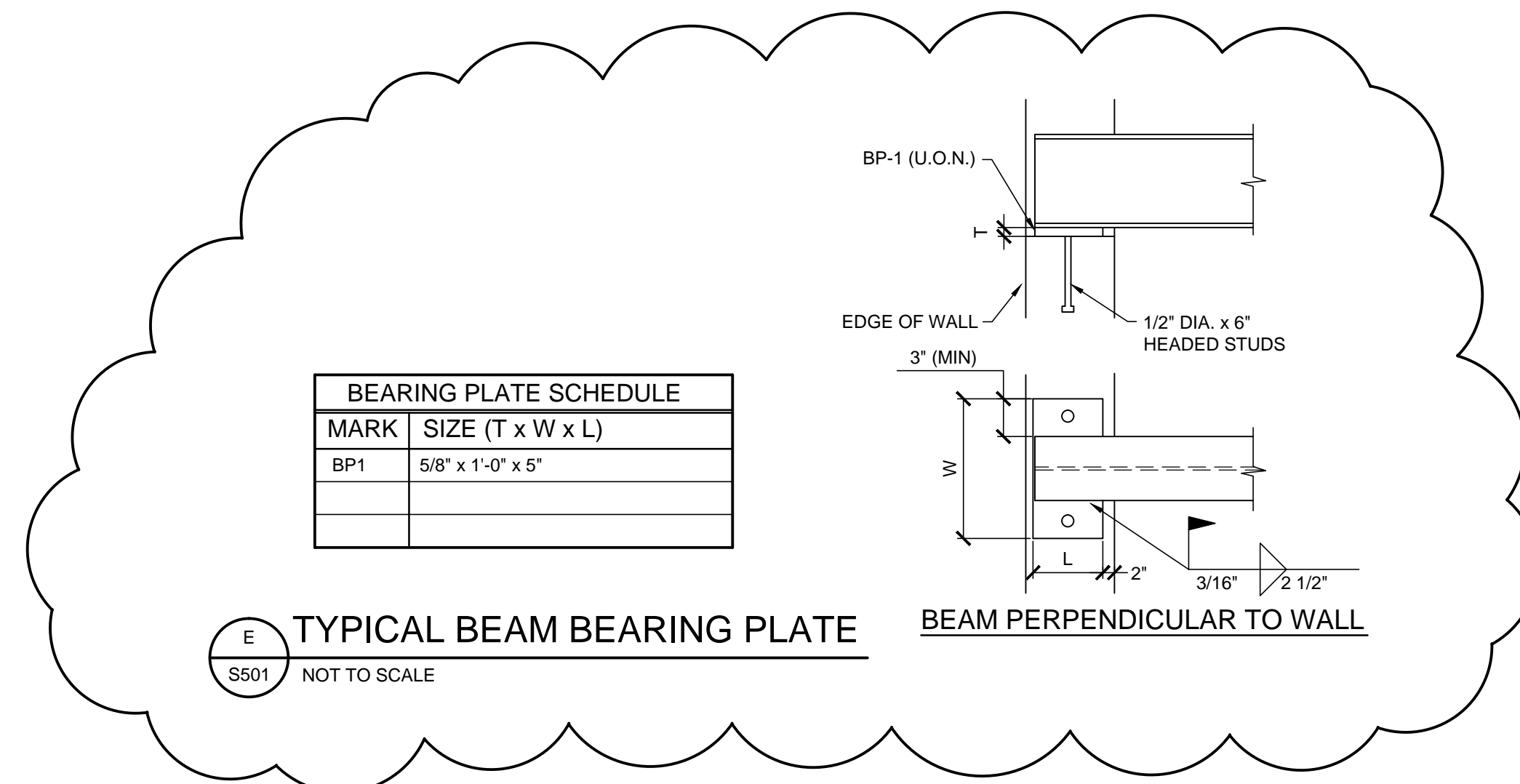
C TYPICAL RE-ENTRANT CORNER REINFORCING
S501 NOT TO SCALE



SEE SECTION 5/S301 CMU FROST WALL CONSTRUCTION

NOTES:
1. SEE ARCHITECTURAL DRAWINGS FOR ALL HANDRAIL DETAILS AND ALL STAIR NOSING, RAKE, RISE AND RUN REQUIREMENTS.

D TYPICAL EXTERIOR CONCRETE STAIR ON GRADE
S501 NOT TO SCALE



E TYPICAL BEAM BEARING PLATE
S501 NOT TO SCALE

MASONRY WALL LINTEL SCHEDULE			
MARK	SIZE	SPAN	REMARKS
CP1	4\"/>		(1) LINTEL PER EACH 4\"/>
L1	L4\"/>	UP TO 4'-0\"/>	(1) ANGLE PER EACH 4\"/>
L2	L5\"/>	4'-0\"/>	(1) ANGLE PER EACH 4\"/>
L3	L6\"/>	6'-0\"/>	(1) ANGLE PER EACH 4\"/>
L4	W8x24 + 3/8\"/>		

NOTES:

- PROVIDE LINTELS AS NOTED ON THE STRUCTURAL PLANS AND PER THE SCHEDULE ABOVE. CONTRACTOR SHALL ALSO PROVIDE LINTELS OVER ALL OPENINGS FOR MECHANICAL DUCTS, LOUVERS, RECESSES, AND OTHER OPENINGS GREATER THAN 12\"/>
- ALL LINTELS SHALL HAVE 8\"/>
- FOR DIMENSIONS AND LOCATIONS OF MASONRY OPENINGS, SEE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.
- ALL LINTELS IN EXTERIOR WALLS SHALL BE HOT DIP GALVANIZED CONFORMING TO ASTM A123.
- ALL ABRADED AREAS AND FIELD WELDS SHALL BE FIELD COATED WITH A COLD GALVANIZING COMPOUND CONFORMING TO ASTM A780.
- TYPICAL PLATE WIDTH SHALL BE 1\"/>

VE REVISIONS	11/1/17

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 MONTGOMERY TOWNSHIP, FRANKLIN COUNTY, PA

TYPICAL DETAILS

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DATE : 08/10/17
 SCALE : AS NOTED
 JOB NO. : 16038
 SHEET
S501

BID SET